

CONSOLIDATION MECHANICS REVIEW (41)

A Review of key points

B Worked examples

4/25/18

GG454

1

II Review of key points

A Heat flow analogous to
flow of fluid during
consolidation

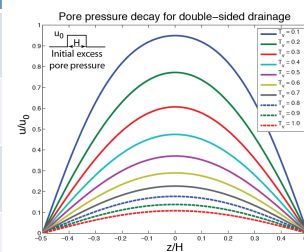
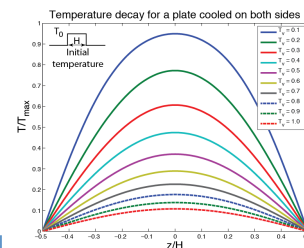
$$\frac{\partial T}{\partial t} = \alpha \frac{\partial^2 T}{\partial z^2} \quad \frac{\partial u_{\text{excess}}}{\partial t} = C_v \frac{\partial^2 u_{\text{excess}}}{\partial z^2}$$

Heat flow	Fluid flow
T Temperature	u_{excess} Excess pore pressure
t Time	t Time
α Thermal diffusivity	C_v Coefficient of consolidation
z position	z position

4/25/18

GG454

2



Here $T_v = t^* = t/(H^2/C_v)$

II Review of key points

B Darcy's law $q = \frac{Q}{A} = -k \frac{\partial H}{\partial x}$

where

q = flux (m/sec)

Q = discharge rate (m³/sec)

A = area (m²)

k = hydraulic conductivity (m/sec)

$\partial H / \partial x$ = head gradient (dimensionless)

4/25/18

GG454

3

II Review of key points

C Darcy's law (alternative form) $Q = -kA \frac{\Delta H}{\Delta L}$

where

Q = discharge rate (m³/sec)

k = hydraulic conductivity (m/sec)

A = area (m²)

ΔH = change in head (m)

ΔL = length of flow path (m)

k (hydraulic conductivity) is a function of the fluid *and* the porous medium

K (intrinsic permeability) is a function of only the medium

Relationship between hydraulic conductivity and intrinsic permeability

$$k = K \frac{\rho g}{\mu}$$

$$K = k \frac{\mu}{\rho g}$$

where

k = hydraulic conductivity (m/sec)

K = intrinsic permeability (m²)

ρ = density of the fluid (kg/m³)

g = acceleration due to gravity (m/sec²)

μ = dynamic viscosity of fluid [kg/(m·sec)]

4/25/18

GG454

4

II Review of key points

B Controls on 1-D consolidation time (t_c) of soils and sediments

- 1 The time for consolidation should scale with the volume of water squeezed out of the soil
- 2 The volume of expelled water is the product of three terms
 - a Effective stress change ($\Delta\sigma'$)
 - B Compressibility of the solid skeleton (i.e., m_v , the coefficient of volume change)
 - C Volume of the consolidating layer (or height H of the layer)

$$t_c \propto \Delta\sigma' m_v H$$

4/25/18

GG454

5

II Review of key points

B Controls on 1-D consolidation time (t_c) of soils and sediments (cont.)

- 3 The time [for consolidation] should be inversely proportional to how fast the water can flow through the consolidating layer
- 4 From Darcy's law: the velocity of flow is scales with the product of the hydraulic conductivity (k) and the hydraulic gradient
- 5 The hydraulic gradient matches the excess fluid pressure lost within the layer divided by the distance the pore fluid flows (H)
- 6 The excess fluid pressure loss scales with $\Delta\sigma'$

$$t_c \propto \frac{1}{k\Delta\sigma'/H} = \frac{H}{k\Delta\sigma'}$$

4/25/18

GG454

6

II Review of key points

B Controls on 1-D consolidation time (t_c) of soils and sediments (cont.)

- 7 t_c increases with increasing compressibility (i.e., with increasing m_v)
- 8 t_c Increases rapidly with increasing volume (height) of the soil/sediment mass (H)
- 9 t_c Decreases with increasing hydraulic conductivity (k)
- 10 t_c is independent of the magnitude of the effective stress change, assuming m_v is a constant

$$t_c \propto \frac{\Delta\sigma' m_v H}{k \Delta\sigma' / H} = \frac{m_v H^2}{k}$$

4/25/18

GG454

7

II Review of key points

C Dimensional analysis illuminates consolidation time

1 1-D consolidation equation

$$\frac{\partial u_e}{\partial t} = C_v \frac{\partial^2 u_e}{\partial x^2}$$

$$\frac{\text{pressure}}{\text{time}} = [C_v] \frac{\text{pressure}}{\text{length}^2} \Rightarrow [C_v] = \frac{\text{length}^2}{\text{time}}$$

where

C_v = coefficient of consolidation

$$= \frac{k}{\rho_{\text{water}} g m_v}$$

m_v = coefficient of volume change

4/25/18

GG454

8

II Review of key points

- 2 The dimensionless time used in dimensionless expressions for consolidation is found in the same way as in lecture 38

Heat flow	Consolidation
$\frac{\partial T}{\partial t} = \alpha \frac{\partial^2 T}{\partial x^2}$	$\frac{\partial u_e}{\partial t} = C_v \frac{\partial^2 u_e}{\partial x^2}$
$t_c \approx \frac{H^2}{\alpha}$	$t_c \approx \frac{H^2}{C_v}$
$t^* = \frac{t}{t_c} = \frac{t}{\left(\frac{H^2}{\alpha}\right)}$	$t^* = \frac{t}{t_c} = \frac{t}{\left(\frac{H^2}{C_v}\right)}$

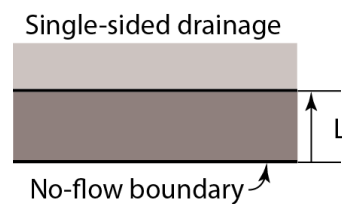
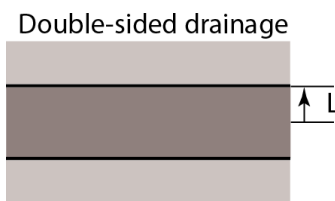
4/25/18

GG454

9

II Review of key points

- D The key length scale (L) is the flow path length
- 1 For double-sided drainage, L = half the layer thickness
 - 2 For single-sided drainage, L = the whole layer thickness



4/25/18

GG454

10

III Calculating consolidation

$$U(t) = \frac{\Delta H(t)}{\Delta H(t \rightarrow \infty)} = \frac{\Delta u_{\text{excess}}}{\Delta u_{\text{excess(max)}}} = 1 - \frac{u_{\text{excess}}}{u_0}$$

- E Consolidation ratio for double-sided drainage of a uniform excess pressure pulse* [for analytical solution see Terzaghi (1943) or Lambe and Whitman (1969)]

1 For $T_v = t/(C_v H^2) > 0.1$

a Excess pore pressure distribution (u) is approximately parabolic

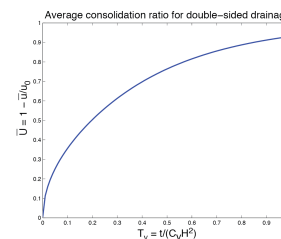
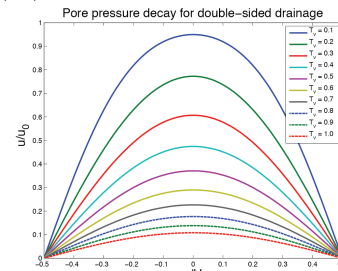
b \bar{U} (ave. consolidation ratio)

i $\bar{U} = 1 - \frac{\bar{u}}{u_0}$

ii $\bar{U} = 1 - \frac{\text{area beneath } u \text{ -curve}}{\text{area beneath boxcar function}}$

iii $\bar{U} \approx 1 - \frac{2}{3} \frac{u_{\text{max}}}{u_0}$

2 ~92% consolidation at $T_v=1$



4/25/18

GG454

11

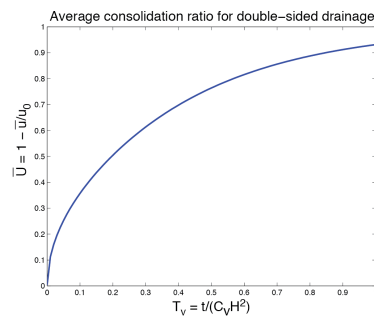
II Review of key points

- E Average consolidation ratio (\bar{U})

1 $\bar{U}(t^*=1) \approx 92\%$

2 $\bar{U}(t^*=3) \approx 99\%$

3 About 92% of the ultimate primary consolidation occurs by the dimensionless time $t^* = 1$



$$t^* = \frac{t}{\left(\frac{x_{\text{max}}^2}{C_v} \right)}$$

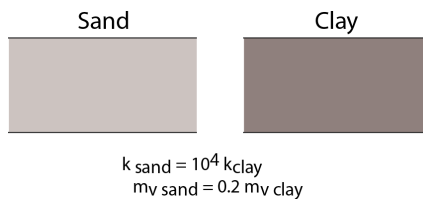
4/25/18

GG454

12

III Examples

A Consider layers of clay and sand, each 10' thick, and suppose that the coefficient of volume change ("compressibility") of the sand is 1/5 that of the clay, and the permeability of the sand is 10,000 times that of the clay.



4/25/18

GG454

13

III Examples

- 1 What is the ratio of the consolidation times of the sand and clay?

$$\frac{t_{\text{clay}}}{t_{\text{sand}}} = \frac{H_{\text{clay}}^2 m_v(\text{clay}) / k_{\text{clay}}}{H_{\text{sand}}^2 m_v(\text{sand}) / k_{\text{sand}}} = \frac{m_v(\text{clay}) / k_{\text{clay}}}{m_v(\text{sand}) / k_{\text{sand}}} = \frac{5/1}{1/10,000} = 50,000$$

- 2 If a 10'-thick layer of clay reaches 90% consolidation in 10 years, how long would it take for a clay layer 40' thick to reach that level of consolidation?

$$\frac{t_{40'}}{t_{10'}} = \frac{m_{v40'} H_{40'}^2 / k_{40'}}{m_{v10'} H_{10'}^2 / k_{10'}} = \frac{H_{40'}^2}{H_{10'}^2} = \frac{40^2}{10^2} = 16 \quad 16 \times 10 \text{ years} = 160 \text{ years}$$

4/25/18

GG454

14

III Examples

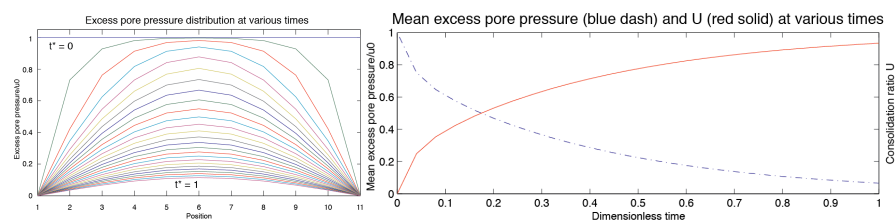
- B How long will a layer of clay take to reach 90% consolidation if the initial excess pore pressure distribution is constant across the layer and the layer is drained from its top and bottom? One dimensionless time unit (t^*) is given by = 1.

4/25/18

GG454

15

C Calculating consolidation for double-sided drainage 4 Matlab script transient_heat4.m



So after one dimensionless time step [i.e. $t^* = t/(H^2/C_v) = 1$], about 92% (i.e., roughly 90%) of the ultimate consolidation will have occurred

4/25/18

GG454

16

C Calculating consolidation for double-sided drainage

3 Matlab function consolidation_v2.m

```
function [U,u] = consolidation_v2(t,Cv,H)
% function [U,u] = consolidation_v2(t,Cv,H)
% Calculates time factor for 1-D consolidation for a uniform excess
% pressure pulse of unit magnitude (i.e., a boxcar function)
% Output arguments
% U = consolidation ratio
% u = excess pore pressure
% Input arguments
% t = time
% Cv = coefficient of consolidation
% H = layer thickness
% Example
% t=0.01:1; Cv = 1; H = 1;
% [U,u] = consolidation_v2(t,Cv,H);
% Calculate time scale Tv
Tv = Cv*(H^2);
z = 0.01*H/2*H;
u = zeros(length(z),length(t));
% Calculate pore pressure (u) by truncated Fourier series (101 terms)
% using eq. (3a) of Terzaghi (1943, p. 274)
for j = 1:length(t);
    for k = 1:101;
        N = k-1;
        n = 2*N+1;
        uu(k) = (4/pi)*(1/n)*sin(n*pi*(z/(2*H)))*exp(-Tv(j)*(n*pi/2)^2);
    end
    u(j,:) = sum(uu);
end
% Calculate consolidation ratio U by trapezoidal integration
U(j) = 1 - sum(u(-:j))/(length(z)-1);
end

% Plot pore pressures at different times
figure(1)
zn = (z-1)/2;
plot(zn,u(:,11),zn,u(:,21),zn,u(:,31),zn,u(:,41),zn,u(:,51),...
    zn,u(:,61),zn,u(:,71),zn,u(:,81),zn,u(:,91),zn,u(:,101));
xlabel('z/H')
ylabel('u/u_0')
title('Pore pressure decay for double-sided drainage')
legend('T_v = 0.1','T_v = 0.2','T_v = 0.3','T_v = 0.4','T_v = 0.5',...
    'T_v = 0.6','T_v = 0.7','T_v = 0.8','T_v = 0.9','T_v = 1.0');

% Plot consolidation ratio
figure(2)
plot(t,U)
xlabel('T_v = t/(C_v H^2)')
ylabel('U = 1 - u/u_0')
title('Average consolidation ratio for double-sided drainage')
end
```

4/25/18

GG454

17

C Calculating consolidation for double-sided drainage

4 Matlab script transient_heat4.m

```
% Matlab script transient_heat4.m
% Solves for 1-D transient heat flow by finite differences
% H = length of longest flow path
a = 11; % # of rows in the T matrix (# of points in space)
b = 26; % # of columns in the T matrix (# of points in time)
numit = 21; % # of iterations at each time step
T0 = 0; % Temperature at one end of rod
T1 = 1; % Temperature at other end of rod

% Initialize the "Temperature matrix"
T=zeros(a,b); % First index is position, second is time step
% Set the initial temperature distribution
T(:,1) = (linspace(T1,T0,a))' % Constant initial temperature

% Solve by finite difference method
for j=2:b; % j is the index for time step
    for k = 1:numit;
        for i = 2:a-1; % i is index for position
            T(i,j) = 0.25*( T(i+1,j-1)+T(i-1,j-1)+T(i+1,j)+T(i-1,j) );
        end
    end
end

% Plot figures
figure(1)
clf
subplot(2,1,1)
plot(T); % Plot columns of T versus index
title('Temperature at various times')
xlabel('Position')
ylabel('Temperature/T_max')
axis([1 a 0 1.05])

subplot(2,1,2)
t_star = [0:b-1]/((a-1)/2)^2
plot(t_star,mean(T),'-b',t_star,1-mean(T),'r')
title('Mean excess pore pressure (blue dash) and U (red solid) at various times')
xlabel('Dimensionless time')
ylabel('Mean excess pore pressure/u0 (or consolidation ratio)')
```

4/25/18

GG454

18

References

- Lambe, T.W., and Whitman, R.V., 1969, Soil Mechanics: Wiley, New York, 553 p.